#### **AN ABSTRACT OF THE THESIS OF**

2000 Kurt Shobe for the Master of Science in Physical Science Presented on November 27,

Title: Analysis of Chloride Plume Migration Based on Aquifer Characteristics, Water Well Pumping Rate, Soil, and Stratigraphic Properties

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A study was performed to determine the fate and transport of chlorides in groundwater on and around a Central Kansas refinery. This was done by sampling monitoring wells over a four year period for chlorides and measuring static water level in these wells to determine groundwater flow. Groundwater modeling was also performed to determine the probable source location(s) for the chlorides noted in groundwater and project changes in chloride concentration and movement expected to occur in the future. **In** addition, chloride source identification was performed after the method of Whittemore to correlate and compare the chemical nature of groundwater taken from the monitoring wells with the postulated source regions. Source identification was also used to verify the results found as part of the groundwater modeling.

Process water for the refinery is taken from four pumping wells located in the northeast part of the refinery facility and operated continuously. These wells pump between 1325 and 3220 liters/min (350 and 850 gpm) and have been pumping long enough that drawdown from these wells has stabilized. The radius on influence for the four wells, taken as a single system, is estimated at 1.3 km (0.8 mile) on the west side and up to 3.4 km (2 miles) on the east side of the facility. Chlorides located within the radius of influence of these are preferentially drawn to the wells over time.

Based on sampling data and a previous study by Whittemore (1997) two primary chloride source regions were identified. One was a shallow source located on the refinery property, consisting of an unlined impoundment that had contained refinery process water until closure and removal in 1994. The other primary source was the Johnson Oil Field, located approximately 3.4 km (2 miles) east of the refinery. The field contains chloride injection wells and formerly had unlined impoundments holding salt water that had been separated from the crude oil.

Groundwater modeling for the refinery and surrounding property confirmed these source regions and determined that the chloride plume was moving toward the refinery at the rate of approximately 67 meters (205 feet) per year. Source identification of groundwater taken from selected wells also verified these source regions and confirmed the modeling results.

Based on this rate of movement, historic chloride data from wells on and around the facility, and the distance from the source to the pumping wells, it is estimated that the chloride plume from the Johnson Well Field reached the onsite pumping wells approximately ten years ago. Transport time from the shallow onsite source and the pumping wells was estimated at slightly over seven years. The modeling results also indicate that the groundwater quality from the pumping wells will continue to decline over time without remedial action.

# **Analysis of Chloride Plume Migration Based on Aquifer Characteristics, Water Well Pumping Rate, Soil, and Stratigraphic Properties**

A Thesis Presented to The Department of Physical Sciences EMPORIA STATE UNIVERSITY

In Partial Fulfillment of the Requirements for the Degree Master of Science

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### **Acknowledgements**

The author would like to extend his deepest appreciation to Dr. James Aber for his patience and assistance during the course of this project. In addition, most special gratitude is extended to Dr. Richard Sleezer, without whom the completion of this project would not have occurred. Special thanks is also extended to Wanda Sponsel for her untiring efforts required to properly format and prepare the document. Finally, sincere appreciation is given to my wife, who supported me throughout the project and allowed me to spend so many hours in its pursuit.

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#### **CHAPTER** 1

#### **INTRODUCTION**

The purpose of this research was to establish the amount, source, and areal extent of chlorides in the vicinity of the subject facility that are affecting or have the potential to affect groundwater quality in that portion of the Equus Beds Aquifer utilized by the facility as part of the refinery operations onsite. To accomplish this, monitoring wells were drilled and used in conjunction with existing wells to sample groundwater beneath the facility and measure its chloride content. The purpose of the groundwater sampling was to delineate the spatial patterns of existing chloride plumes in the vicinity of the refinery. Stratigraphic studies were undertaken to establish the predominant and/or preferential pathways for groundwater and solutes through the aquifer as a function of hydrostratigraphy. In addition, variations in aquifer thickness both on and around the refinery property were mapped to help understand the implications for the zone of influence for pumping wells at the refinery. Results of the stratigraphic work were verified through the use of groundwater modeling on and around the site to determine if the predicted preferential flow paths were indeed influencing groundwater flow. Finally, a chemical signature approach developed by Don Whittemore at the Kansas Geological Survey was used to differentiate between possible sources for chloride pollution measured in groundwater samples from the monitoring wells.

#### Refinery Description and Location 1.1

The facility studied is an 80,000 barrel per day (bpd) crude oil refinery. This facility is located on about 981 acres of land located approximately one-half mile south of the City of McPherson, Kansas **(Figure 1).** Petroleum refining operations began at this facility in 1933 and refining operations have continued to the present day.

Land use surrounding the refinery is predominantly agricultural (cash-grain operations). Exceptions include a radio station (KNGL) and a closed petroleum product terminal (Derby Refining Corporation) located adjacent to the southeast side of the refinery property, a sand pit located a quarter mile south-southwest of the refinery, and the McPherson Airport located a quarter mile northwest of the refinery.

#### 1.1.2 Previous Investigations:

Since 1986, two separate geologic and hydrogeologic investigations have taken place on the refinery property. The following is a discussion of these investigations, including information pertinent to the present study. Previous studies that have been utilized to present the information contained below include:

- "National Cooperative Refinery Association Hydrogeologic Investigation and Monitoring Proposal"; A&M Engineering & Environmental Services, Inc. 1986.
- Hydrogeologic Investigation Report"; EEI Engineering Enterprises, Inc., 1988.

#### CHAPTER 2

### **GEOLOGIC SETTING**

#### 2.1 Topography

The region on which the refinery property is situated is a part of the Great Bend Physiographic Province. Topography in the vicinity of the refinery facility is relatively flat (0-1% slopes) (Rott, 1983). East and north of the facility topography ranges from gently sloping to sloping (1-8% slopes) as a result of the incisement of **Bull** Creek and Dry Turkey Creek into the underlying unconsolidated sediments.



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Figure 1 - Refinery and Surrounding Property (USGS 7.5 min. topo)

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clay, silt, and gravel. The refinery lies on top of the McPherson fonnation that lies within the McPherson Valley which is a bedrock channel cut into underlying Pennian fonnations when the ancestral Smoky Hill River flowed southward into the Arkansas River. The McPherson Formation varies in thickness, but is approximately 55-63 meters (170-190) feet) thick beneath the site. The upper part of the McPherson Fonnation is mainly clay and silty clay with thin (one meter or less) zones with carbonate concretions and silt in the upper 13-23 meters (40-70 ft). The lower part of the McPherson Fonnation is mainly sand and gravel with clay interbeds and tongues. The clay beds in the lower part range from a meter to 6.5-10 meters (20-30 feet) thick. An erosional unconfonnity separates the Pleistocene McPherson Formation from the underlying Permian Wellington Shale. The Wellington Shale is gray, bluish gray, and grayish green, medium soft to hard and calcareous, with thin argillaceous limestone and gypsum interbeds (McElwee, et. al. 1979, Spinazola, et. aI., 1985). Water is scarce in the Wellington, however salt deposits are known to be imbedded in the shale and chloride rich groundwater has been reported from the Wellington (McElwee, et. aI., 1981).

#### 2.2.1 Results of Previous Studies at the Facility

The previous geologic studies performed onsite (see Section 1.1.2) show that the stratigraphic section above the Wellington at the site consists of about 16.5 meters (50 feet) of silt and clay, over 3-7 meters (10 to 20) feet of sand and fine gravel, overlying 10 13 meters (30 to 40 feet) of silty clay with tine sand. The basal unit consists of 7-13 meters (20-40 feet) of sand and gravel with clay lenses. The reports noted changes in stratigraphy from boring to boring, which was indicative of reworked stream deposits.

Soil samples **(Appendix 1)** taken from borings drilled to approximately 7 meters (20 feet) below ground level (bgl), in the vadose zone, reveal  $65\%$  to  $85\%$  clay, with silt comprising the vast majority of the remaining sediments. One remolded sample was tested for hydraulic conductivity and had a value of  $4 \times 10^{-6}$  cm/s. This indicates that recharge from the overlying vadose zone is minimal.

#### 2.2.2 Geologic Information Gained as Part of the Current Study

#### 2.2.2a Field Activities:

As part of the current study, sixteen (16) monitoring wells were drilled on and around the refinery facility in September and October 1999. Drilling logs for these wells can be found as part of **Appendix** 1. Six of these wells were completed in the upper part of the saturated zone (36-41 meters or 110 to 124 feet bgl), with the remaining ten completed to Wellington bedrock (between 49-60 meters or 150 to 183 feet bgl). An experienced field geologist was present during all drilling activities, with oversite provided by the author.

The shallow wells were advanced utilizing hollow stem augers, which allowed relatively undisturbed soil samples to be taken. As part of the drilling activities, these were collected and analyzed at five (5) foot (1.6 meter) intervals from ground surface to completed depth. For the deep wells, mud rotary drilling technique was utilized as hollow stem augers could not be advanced to bedrock. Soil samples from these wells were also collected at five (5) foot intervals. Due to the nature of the drilling technique, these samples were highly disturbed. However, the samples taken are felt to be fairly representative of the composition of the material at each sample depth, in particular those

taken in the saturated zone. Most of the saturated zone samples were described as sand, with sand and gravel at lower depths. Samples taken from the saturated zone that contained appreciable clay content could be readily distinguished, even though these samples had been reworked by the drilling activities.

#### 2.2.2b Stratigraphic Study

The stratigraphic sections across the site **(Figures 3 and 4)** were generated utilizing data from the monitoring wells completed in September and October 1999. The sections were made by constructing a N-S and E-W line across the site, respectively, which intersected as closely as possible the greatest number of wells. Distances between well logs were based on surveyed data, and the distances between wells on the crosssection were based on the interpolated location of each well placed on the transect line.

The stratigraphy revealed by the well logs shows a lithology primarily composed of alternating layers of sand and clay, with weathered shale and shale bedrock. Review of the N-S and E-W cross sections reveals a fairly consistent lithology across the study area, in good agreement with the results noted in Section 2.2 above. Thin, localized clay stringers and silt stringers are located at various depths within the sand; however the cross-sections do not indicate that any of these clay and silt lenses would be a signi ficant deterrent to vertical groundwater migration.

Review of the E-W cross-section also shows that the saturated thickness increases westward underneath the refinery. Monitoring well 110D, the easternmost monitoring well, shows a saturated thickness of only about 16 meters (50 feet), while the recovery well (RW) on the wcsternmost side of the cross-scction indicates a saturated

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thickness of nearly 26 meters (80 feet). **In** addition, it can be clearly seen that the sand content generally increases westward. The middle clay layer decreases in elevation eastward to the point that the clay intersects the static water level elevation. *However,* the clay layer pinches out west of 20S, leaving a continuous column of sand from approximately 475 meters (1450 feet) above mean sea level (msl) to bedrock at 436 meters (1330 feet) msl. **Figure 5** illustrates a plan view of the North-South and East-West cross sections for reference as to location of the cross sections in relationship to the facility.



Figure 3 - East - West Cross Section







Figure 5 - North-South & East-West Cross Sections; Plan View

#### CHAPTER 3

#### **AQUIFER IDENTIFICATION AND PROPERTIES**

The sand and gravel units in the lower part of the Pleistocene McPherson Formation comprise the aquifer beneath the site. This aquifer is referenced as the "Equus Beds". The "Equus Beds" aquifer is an unconfined type. The hydraulic characteristics of the "Equus Beds" aquifer at the refinery have been detennined from tests conducted on the monitoring wells and from published reports.

3.1 Results of Previous Investigations:

#### 3.1.1 Aquifer Characteristics:

As noted above, the facility lies above the McPherson channel, an area characterized by deposits from ancient stream channels. Many of the sediments comprising the older part of the McPherson Formation, into which the McPherson channel was cut, were deposited by southward flowing streams. Streams contributing to the deposition entered from the north, northeast, and northwest. Smaller streams entered from the east and northeast. All streams were nearly at grade, resulting in the copious deposition of silt in the floodplain (EEl report to NCRA, 1986).

The saturated zone, known as the "Equus Beds" historically began at a depth of approximately 16 meters (50 feet) bgl. This upper part of the aquifer consisted of the 3 to 6 meter (10 to 20 foot) section of sand directly underlying the top 16 meters (50 feet) of clay and silt. Pumping at the refincry has reduced this zone and it is no longer found in the area of the refinery.

The saturated zone currently begins roughly 25 to 29 meters (75 to 90 feet) bgl

at the site and varies in thickness due to changes in bedrock topography across the site. A maximum saturated thickness of 42 meters (128 feet) has been reported on the west side of the refinery and a minimum of24 meters (74 feet) on the east side.

Chloride sampling of the four water supply wells was performed as part of the 1986 study and results of this sampling is given as **Figure** 6. It is evident from the results illustrated that chloride contaminants had not affected any of the pumping wells at that time.

#### 3.1.2 Aquifer Hydraulics:

Groundwater flow in the area around the Refinery is strongly influenced by the presence of four large pumping wells located in the northeast comer of the facility **(see Figure 5 and maps, Appendix** 4). The area of influence for these pumping wells was estimated at 1.3 km (0.8 mile) along the western edge and postulated to be at a similar range on the east side of the pumping wells. Both reports indicated that the pumping wells have established a dominant cone of depression influencing the groundwater flow in and around the refinery area. Groundwater flow absent of the pumping wells was reported to be toward the northeast at a low gradient, however no source is given to validate this assumption.

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Figure 6 - Chloride Levels in the Pumping Wells, September 1986 (From A&M,  $1986$ 



#### GROUND WATER QUALITY

\*No Analysis





Figure 7 - Transmissivity Data (From A&M, 1986)

Transmissivity data was collected from a number of the wells drilled in 1986. Values ranged from a high of 1545  $m^2/d$  (124,600 gpd/ft) on the far west side of the property to a minimum of 370 m 2 /d (29,900 gpd/ft) on the east side (See **Figure** 7). Values from wells on the north side of the refinery indicated a transmissivity of approximately 1240 m<sup>2</sup>/d (100,00 gpd/ft). No information is available for the south side of the facility. Transmissivity under the refinery was estimated to range from 248 to 620  $\text{m}^2/\text{d}$  (20,000 to 50,000 gpd/ft). Pump tests were performed for selected wells as part of the study done in 1988. Results showed that the  $#10$  water well was tested to have a transmissivity of 310 m<sup>2</sup>/d (25,000 gpd/ft) and the #9 water well was tested to have a transmissivity of 646 m<sup>2</sup>/d (52,100 gpd/ft). The previous studies do not present any information which might explain the variation s in the transmissivity values.

Permeability in the #9 water well was estimated at  $1 \times 10^{-1}$  cm/s. Water usage from the wells was estimated at an average of 7575 l/min (2000 gal/min), resulting in a yearly usage of 3226 acre-ft per year. The reported hydraulic conductivity values range from 3.7 to 14 m/d (90 to 340 gpd/ft<sup>2</sup>). The vertical gradient was calculated for test wells completed in the area of the land treatment unit, located on the south side of the facility. The report indicated that vertical gradients was downward in the vicinity of 3 of the 4 wells, but were upward in the vicinity of "well 3", near the present location of LF-3.

#### 3.2 Results from the Current Study

### 3.2.1 Aquifer Characteristics

As part of the current study, regional groundwater flow was studied to determine the potential for off-site migration of chlorides onto the refinery property. This information can be used in conjunction with the radius of influence for the four pumping wells to determine if chlorides from off-site would be preferentially transported to the pumping wells.

#### 3.2.2 Groundwater Flow

**Figure 8** shows the configuration of the water table in the vicinity of the study area. It was created using forty-five (45) water table elevation control points downloaded from the Kansas Geological Society (KGS) Wizard database. The regional groundwater flow around the refinery property is complex. The predominant flow directions in the area around the refinery are to the north, northwest, or west. **In** the immediate vicinity of the refinery it can be seen that groundwater flow converges toward the pumping wells onsite. Outside of the zone of influence of the pumping wells, it may also be moving west, toward the center of the McPherson Channel located west of the refinery.

It can also be seen that there is a groundwater divide (or a groundwater high) to the south of the refinery. Groundwater north of this divide flows preferentially north toward the Smoky **Hill** River; groundwater south of this divide flows south towards the Little Arkansas River. The pumping wells at the refinery will capture water moving west toward the center of the McPherson Channel, and will also capture water from the south which is preferentially moving north due to the groundwater divide. Water on the west side of the refinery which is captured by the pumping wells will be diverted from a northnorth-west flow direction to an easterly flow direction. Likewise, water in the aquifer located north of the refinery captured by the pumping wells radius of inlluence will be diverted approximately 180 degrees from the regional flow direction.

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Figure 8 - Regional Groundwater Flow (ft MSL) 1980.

#### 3.2.3 Saturated Thickness and Bedrock Topography

To better establish the saturated thickness within the study area, an interpolated grid of bedrock topography (Figure 9, taken from Sleezer, 2000) was prepared which encompassed a two square mile area centered on the refinery. This study area was further subdivided into the refinery study area and the chloride study area. The chloride study area was the extent of the area modeled using Groundwater Vistas<sup>TM</sup>, a windows based groundwater model based on the USGS groundwater model ModFlow.



Figure 9 - Interpolated Bedrock Topography (ft MSL)

A review of Figure 9 shows that the bedrock elevation within the map area varies significantly from west to east. Based on this figure, bedrock elevation on the east side of the chloride study area was estimated at 440 meters (1340 feet) above mean sea level (msl), while the bedrock elevation on the west side of the chloride study area was measured at 420 meters (1280 feet) msl.

The water table elevation (Figure 10) across the chloride study area was determined by measurement of static water level elevation in the refinery wells and



**Figure 10- Static Water Level Elevations (after Sleezer, 2000)** 

published values taken from the aforementioned KGS Wizard database. This indicates that static water levels vary from 462 meters (1410 feet) msl on the west side of the refinery to 456 meters (1390 feet) instrom the east side of the refinery.

Using these two views, the variability in saturated thickness across the chloride study area can be shown to vary from a minimum of approximately 16.4 meters (50 feet) on the east to a maximum of  $42.6$  meters (130 feet) on the west. This differential in saturated thickness indicates that the radius of influence for the pumping wells (taking all four wells as essentially one system) could be significantly greater on the east side of the refinery as opposed to the west side. This assumes tbat roughly an equal volume of water is pulled from the aquifer by the pumping wells on all sides.

#### 3.2.4 Radius of Influence for the Pumping Wells

This information is significant when attempting to determine the potential source areas for the chlorides noted in water samples taken from the pumping wells. A larger radius of influence means that the area available for potential sources to be captured by the effects of the pumping wells is greater. In addition, a larger radius of influence on the east side of the refinery is aided by the fact that the regional flow on the east side of the refinery is predominantly westerly. Therefore, the radius of influence coupled with the regional flow will have an additive effect when determining the total area east of the refinery from which potential sources of chloride can be drawn from. Unfortunately, there is an insufficient number of wells on the east side of the refinery to make an accurate determination of the true radius of influence in this direction.

However, it was possible to estimate radius of influence on the west side of the refinery. **Figures 11-18** illustrate static water level elevation vs distance for all four of the pumping wells. As the wells operate 24 hours per day every day, it was not possible to establish the static water level elevation of the wells absent the effects of pumping. Therefore, it was not possible to establish the ambient static water level for comparison to measured values in monitoring wells at various distances from the pumping wells. Normally, this ambient value would be compared to the measured values taken while the pump test was ongoing and after the measurements had stabilized when attempting to discern the radius of influence.

To approximate the static water level recovery rate with distance, a best fit line was added to each graph in **Figures 11-14**. The radius of influence was estimated as the point at which the measured data intersects the best fit, or hypothetical recovery. It was

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postulated that the point at which the measured recovery rate was less than the hypothetical recovery rate would represent the extent of the pumping wells influence.



Figure 11 - Estimated Radius of Influence for Water Well 9



Figure 12 - Estimated Radius of Influence for Water Well 10


Figure 13 - Estimated Radius of Influence for Water Well



Figure 14 - Estimated Radius of Influence for Water Well 3

This assumption can be tested to a degree by comparisons between the intersection point and the approximate static water level elevation believed to be outside the radius of influence of the pumping wells. As noted earlier, the static water level elevation on the west side of the study area is approximately 1410 feet msl. The distance between the edge of the study area and the pumping wells is assumed to be at or beyond the effective radius of influence of the four well complex. This distance is approximately 1312 meters (4000 feet) from pumping well #9, which is in general agreement with the estimated radius of influence of 1.3 km (0.8 mile) given as part of a previous hydrogeologic investigation at the site (see Sec. 1.1.1 above).

Based on analysis of these graphs, the radius of influence for the four pumping wells were estimated as:



**Figures 15-18** are logarithmic distance drawdown graphs with a curve fitted to the measured data and taken out five log periods to estimate changes past the available distances. Correlation coefficients were calculated for each curve and are also shown on the graphs  $(R^2)$ . This curve-fitting technique was utilized to estimate the static water level elevation changes with distance beyond the available data. These graphs suggest that the radius of influence may be somewhat greater than that indicated by **Figures 11-14.** 

As can be seen from **Figures 15-18** below, the curves for water wells #3, #8,

and #10 continue to increase slightly beyond the available data. As this is a theoretical curve, it is impossible to establish the point on the curve which intersects the actual static water level elevation. However, if 462 meters (1410 feet) msl is used as an estimate of this elevation, theses graphs indicate that the radius of influence for the pumping wells would be approximately:





Figure 15 - Distance Drawdown Radius of Influence for Water Well 9



**Figure 16 - Distance Drawdown Radius of Influence for Water WellS** 



Figure 17 - Distance Drawdown Radius of Influence for Water Well 10



Figure 18 - Distance Drawdown Radius of Influence for Water Well 3

#### 3.2.5 Storativity, Transmissivity, and Hydraulic Conductivity



Figure 19 - Monthly Water Usage

Figure 19 illustrates the average pump rate for the water supply wells during 1998. Values shown here represent the average daily pump rate for each month of the year and each well. Using the Radius of Influence information contained above and the known average pump rate taken from refinery records **(Appendix** 2), aquifer characteristics were derived using the methods given as part of "Basic Groundwater Hydrology"; USGS Water Supply Paper 2220, Heath, 1991.

**Table 1 - Average Pump Rate (gpm) for the Four Pumping Wells** 

Water Well #3		Water Well #8		Water Well #9		Water Well #10	
Mean	376.01 Mean		679.67 Mean		809.46 Mean		362.06
Median		380.00 Median		680.00 Median		840.00 Median	376.50
Mode	400.00 Mode		700.00 Mode		900.00 Mode		400.00
Standard Dev.	33.40	Standard Dev.		49.58 Standard Dev.		107.04 Standard Dev.	83.83
Minimum	40.00	Minimum		542.00 Minimum		$195.00$ Minimum	135.00
Maximum		430.00 Maximum		$812.00$ Maximum		970.00 Maximum	750.00
$C$ ount	338.00 Count		338.00 Count		338.00 Count		338.00

**Table 2 - Average Depth to Water (bgl) for the Four Pumping Wells** 

Water Well $\#3$		Water Well #8		Water Well #9		Water Well $#10$	
Mean	118.03	Mean	110.11	Mean	110.39	Mean	116.05
Median	118.00	Median	111.00	Median	110.00	Median	119.00
Mode	119.00	Mode	112.00	Mode	110.00	Mode	120.00
Standard Dev.	1.86	Standard Dev.	4.25	Standard Dev.	2.21	Standard Dev.	13.17
Minimum	105.00	Minimum	98.00	Minimum	103.00	Minimum	17.00
Maximum	121.00	Maximum	116.00	Maximum	118.00	Maximum	121.00
Count	123.00	Count	123.00	Count	123.00	$C$ ount	123.00

The Jacob Time-Drawdown analysis and Jacob Distance-Drawdown analysis was used to estimate the aquifer properties. Results shown in **Table 2** above were used to determine the average drawdown at each pumping well. Time-Drawdown analysis was used to determine the time required for the cone of depression to reach steady state. This calculation is based on the formula developed by Jacob (1950):

$$
t_c = 7{,}200r^2S/T
$$

Where  $t_e$  = the time in minutes at which steady state conditions exist,  $r =$  the outermost distance from the pumping well (in meters),  $S =$  the estimated storage coefficient (dimensionless), and *T* is the estimated transmissivity, in square meters per day.

Values used to determine the time to reach steady state were determined as follows:

The estimated Storage Coefficient  $(S)$  of  $10^{-3}$ was taken from Heath (1991). The outermost distance from the pumping well  $(r)$  was taken as 1304 meters (3975 feet), which was the measured distance from WW#9 to monitoring well ZL-5

Transmissivity was calculated from the formula given by Jacob (1950):

$$
T = 70Q/\Delta S
$$

Where  $Q =$  the pumping rate (in gal/min); taken from **Table 1** above, and  $\Delta S =$  the drawdown across one log cycle (in feet), taken from Figures **11-14** above. This equation is specifically tailored for English units; therefore the English unit results were then translated into metric values.

Transmissivity for well #9 is then:

 $T = 70(809.46$  gpm $)/2.77$  feet  $T = 1900 \text{ m}^2/d (20,455 \text{ ft}^2/day)$  Transmissivity was similarly calculated for each pumping well and is listed on Table 3 below. The calculated values and the averages are:



Average of all wells 776 m<sup>2</sup>/d (8355 ft<sup>2</sup>/day)

The time required to reach steady-state conditions, using WW#9, is then:

 $t_c = 7{,}200r^2S/T$  $t_c = 7,200(1404^2 \text{ m})(10^3)/(1900 \text{ m}^2/d)$ *te* =5537 *minutes* 

Time was similarly calculated for each pumping well and is listed on Table 3 below. Using the information calculated above, storativity could be calculated using the formula by Jacob (1950):

$$
S = T t / 640 r_o^2
$$

Where S = Storativity, T = Transmissivity, t = time, and  $r<sub>o</sub>$  = distance from the pumping

well to the point where the ambient static water level elevation at the pumping well intersects the zero-drawdown line (see Figures 7-10). This formula is also designed specifically for English units; however as Storativity is a unitless valuc, no unit conversion is necessary. Again using WW#9, the storativity is calculated as:

# $S = (20,455 \text{ ft}^2/\text{day})$ (5537 min) / 640 (1900 feet)<sup>2</sup>  $S = 0.05$

Storativity was similarly calculated for each pumping well and is listed on Table 3 below. Using the transmissivity values calculated above and the measured aquifer thickness known from the onsite monitoring wells, it was possible to calculate the hydraulic conductivity  $(K)$  at the pumping wells. The formula used to determine hydraulic conductivity was taken from Heath (1991) and is:

## $K = T/b$

Where  $K =$  hydraulic conductivity (m/day),  $T =$  transmissivity, (m<sup>2</sup>/d) and  $b =$  aquifer thickness (m). For water well #9, the hydraulic conductivity was calculated as:

$$
K = (2736 \text{ m}^2/d) / (31.2 \text{ m})
$$

$$
K = 70.9 \text{ m/day}
$$

Hydraulic Conductivity was calculated similarly for all wells and the values are listed in Table 3 below.



# Table 3 - Aquifer Characteristics

The average value for hydraulic conductivity is in good agreement with published reports (McElwee, et. al, 1979, Spinazola, et. al., 1985, Williams, et. al, 1949) which indicate that a hydraulic conductivity in the range of 33 m/d (100 ft/day) is expected at the site. Heath (1991) shows hydraulic conductivity for medium to coarse sand ranging from 3.3 to 33 m/d (10 to 100 ft/day), again in good agreement with the calculated numbers.

In an effort to verify the hydraulic conductivity values calculated above, the vertical potential gradient (VPG) was calculated for a number of the nested well clusters located across the study area. Hydraulic conductivity was then calculated at each well cluster.

The formula used for VPG was taken from Darcy's law (Heath 1991) and is:

$$
VPG = d\frac{h}{dl}
$$

Where

# $VPG =$  *shallow well water elevation (ft) – deep well water elevation (ft) mid-point of the shallow well screen(ft) – mid-point of the deep well screen (ft)*

Hydraulic conductivity is also found using Darcy's equation (Heath 1991):

$$
Q=KA(dh/dl)
$$

Where  $\boldsymbol{Q}$  = the pump rate (assumed to be 1 l/m) and  $A$  = the cross-sectional area (also assumed to be 1 m<sup>2</sup>), K and  $dh/dl$  are as above. Rearranging terms and eliminating the unit values gives

#### $K = dl/dh$

Giving hydraulic conductivity as the inverse of VPG, assuming vertical flow of 1 liter per minute through an area of I square meter. Results for the calculations illustrated above are given below as part ofTable 4

Top of casing (TOC) elevations were established by a registered land surveyor at the time the wells were completed. MSL SWL elevations were measured on November 15 and 16, 1999. Screen length and total depth measurements were taken from well logs. The average hydraulic conductivity calculated using the nested well clusters assuming unit values for area and pumping rate was  $29.7 \text{ m/d}$  (90.6 ft/day), which is in good agreement with the average calculated hydraulic conductivity value of  $30.4 \text{ m/d}$  (92.7) ft/day) found using the pumping wells.

Monitoring Well	MSL TOC	MSL SWL	Screen Length	Total Depth	MSL. Total Depth	Vert. Gradient	Estimated K $({\rm ft/d})$
						$({\rm ft}/{\rm ft})$	
10S	1487.240	1406.720	25.000	120.000	1367.240	0.201	4.969
10D	1487.510	1406.070	20.000	171.000	1316.510		
11S	1490.360	1405.990	30.000	88.000	1402.360	$-0.005$	214.000
11D	1491.640	1406.220	20.000	167.500	1324.140		
108S	1497.310	1405.820	25.000	110.000	1387.310	$-0.006$	159.909
108D	1497.220	1405.930	20.000	170.000	1327.220		
111S	1499.530	1400.050	25.000	110.000	1389.530	$-0.020$	50.523
111D	1499.190	1400.700	30.000	185.000	1314.190		
21S	1489.950	1406.080	30.000	83.000	1406.950	0.022	45.108
21D	1490.750	1404.690	20.000	173.000	1317.750		
<b>20S</b>	1488.500	1407.130	25.000	110.000	1378.500	0.034	29.147
20D	1488.270	1406.110	90.000	182,000	1306.270		

Table 4 - Vertical Potential Gradient (dh/dl) and Hydraulic Conductivity (ft/d)

Note: Vertical gradient was calculated as dh/dl, using the midpoint of the screened interval to find dl. K was estimated as the inverse of VPG (dl/dh); assuming a unit value of 1 for Q and A. SWL elevations taken on November 15 and 16, 1999 Average  $K = 90.6$  fl/d, average  $VPG = 0.004$ 

# 3.2.6 Static Water Level Elevation and Water Table Configuration

As noted in **Figure** 8 above, the regional groundwater flow in the area of the refinery is complex. **Figure 20** below shows the groundwater flow confined to the area of the chloride study. This figure was generated from water level measurements taken in November 1999 and includes the sixteen (16) newly completed wells onsite. A total of 113 wells **(Appendix** 3) were used to establish the water table configuration map shown below. As can be seen from the figure, water table elevations generally decrease from west to east across the refinery proper. The highest water table elevation is approximately 463 m (1410 feet) msl on the west side of the refinery. The water elevation decreases to a low of approximately 458 m (1395 feet) msl on the east side near the pumping wells.



Figure 20 - November 1999 Water Table Configuration (ft MSL)

It can also be seen from the figure that there are very few data points available on the east side of the facility. Therefore, the elevations shown on this figure for the east side must be interpreted with caution. While available information (Whittemore, 1997) does indicate that the trend shown here is accurate, there is insufficient data available onsite to make definitive conclusions.

According to Kansas Geological Survey (KGS) Open File Report  $# 79-7$ , regional ground water flow direction in McPherson and vicinity is to the north-northwest This is the natural direction of flow as determined by the KGS, however **Figure 20** above clearly indicates that the pumping wells have a significant effect on local direction of flow in the area of the refinery.

#### CHAPTER 4

#### **FIELD STUDY RESULTS**

## 4.1 Introduction

To discern the extent of the chloride levels across the site, samples were taken from onsite monitoring wells and analyzed to determine chloride concentrations. These samples were analyzed by the NCRA laboratory, which is a state certified lab. Separate sampling events were performed on August 1995, January 1999 and November 1999. Chloride concentration maps based on the results of these sampling events are included below and as **Appendix 4** and are labeled August 1995, January 1999 and November 1999.

For the August 1995 and January 1999 sampling events, selected wells were sampled based on location throughout the refinery. Wells for sampling were selected based strictly on location, without regard to depth and/or screened interval. As can be seen from the maps, the January 1999 event was considerably more comprehensive than the August 1995 event, and served in part to verify results noted as part of the earlier event.

Sources for chlorides within the process water can come from a number of process related areas. One of the major contributors to chloride concentration is saltwater naturally contained in the crude. As a general rule, crude is mixed with brine water in the formation it is taken from by the producing well. Separation of the crude/brine mixture is perfonned at the tank battery in the field and the crude is then transported to the refinery. However, separation is never complete and a small percentage  $(1\%$  to  $2\%)$  of the brine remains emulsified in the crude.

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Another reason why elevated chlorides may be found in the effluent is that the process water is recirculated three to four times before final release to the aeration pond (See Appendix 4). In addition, pond water can also used for process water. Consequently, the chloride levels can be concentrated over time before eventual release to the receiving stream.

Finally, it was discovered in 1995 that the water pumped by the water supply wells from the Equus Beds aquifer contained elevated levels of chloride. At the time of the initial study in August of 1995 the source of the elevated chlorides in the water from the supply wells was unknown. However, it was felt that the unlined lime ponds and/or water retention ponds located on the northwest side of the refinery (see maps Appendix 4) could have been a factor. Due to the radius of influence for the pumping wells, the chlorides could be drawn in from the area of the ponds once migration to groundwater had occurred.

However, the water would need to have migrated through approximately 16 m (50 feet) of low permeability clay. Previous analysis of the vadose zone material indicated a penneability on the order of I x 10-7 *cmls,* which is the EPA standard required for Municipal Solid Waste Landfills. Therefore, without a preferential pathway, a long time period, or some other unknown variable, the possibility that migration to the groundwater had occurred from this source was initially thought to be small.

# 4.2 Results and Conclusions Drawn from the August 1995 Event

The August 1995 event consisted of the sampling and analysis of eightecn (18) monitoring wells and water supply wells located primarily on the north and cast sides of

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the refinery. Of note, almost no sampling was done in the refinery operations area proper nor in the area of the refinery tank farm. A contour map of the August 1995 chloride levels is given below as **Figure 21.** 

The August 1995 event had suggested that one potential source region for chlorides was the old Lime Ponds and possibly the water treatment ponds, both located on the northwest side of the refinery (see maps, Appendix 4). The Lime Ponds had been utilized for treatment of process water for a number of years until their removal in 1995. The water treatment ponds serve as the repository of both refinery stormwater runoff and treated process water before discharge through the National Pollutant Discharge Elimination System (NPDES) effluent to Bull Creek. Neither the Lime Ponds nor the treatment ponds were lined.

Historically, the process water was treated for hydrocarbons and other priority pollutants before discharge, as required by the NPDES pennit. However, chloride has not been listed as a priority pollutant; therefore no treatment for chloride has been done. If the chlorides were able to migrate through the vadose zone, it was possible that these ponds could be a primary source of the chlorides found in the water supply wells. The ponds represented areas where at least the top few feet of soil and vadose zone material had been disturbed. Consequently, if no other changes were caused by construction, at least the pathway to groundwater was shortened. It is also conceivable that disturbing the native material in the vicinity of the ponds may have had some effect on the material below the ponds. There were no other sources suggested by the results of the August 1995 samples.

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Figure 21 - August 1995 Chloride Levels

#### 4.3 Results of January 1999 Sampling Event

The January 1999 sampling event was undertaken primarily to verify and expand the results of the August 1995 event. Consequently, more wells were sampled and the sample locations were chosen with these goals in mind. A total of twenty-five (25) wells were included in this analysis. A contour map of chloride concentrations is included below as **Figure 22.** 

Analysis of the results (Appendix 3) do verify, in part, what was noted as part of the earlier event. The map indicates a narrow area approximately 100-130 m (300-400) feet wide either side of a line beginning at monitoring well ZL-1 and ending at WW-8 where chloride concentrations are at or above 150 mg/l. Sampling results indicate that background levels should be generally in the 50 mg/I to 150 mg/l range.

The shape of the plume indicated the possibility of a point source much smaller than the retention pond, and suggests that the area around the Lime Ponds may be the primary source. This is supported by the chloride levels noted in LP104 (71.4 mg/l) vs. LP102 (461.6 mg/l). If this is the case, then it would be expected that the chloride levels will slowly decline over the next few years, finally reaching and equilibrium level equivalent to the rate of influx from the vadose zone. This can be stated as the lime ponds have been removed, therefore, no source renewal is occurring there.

As the water retention pond extends north of the Lime Pond wells. it would be expected that any leakage from the pond might be noted in LPI04 at comparable levels to LP102. While this was not noted in 1999, this circumstance was found in 1995, which could indicate generally decreasing concentrations coming from the pond.

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Figure 22 - Chloride Levels, January 1999

Another possibility considered at that time was that the radius of influence for the water supply wells, in particular WW-6 and WW-lO, allows only a part of the plume from the big pond to be captured and moved southeast. Using the KGS number for hydraulic conductivity (100 ft/day), the transport time for water from the pond to WW-6, approximately 2,000 feet, is calculated as follows (after Heath, 1991).

#### $V = KI/N$

Where *V* is the velocity, in m/day, *K* is the hydraulic conductivity, in m/day, *I* is the hydraulic gradient, in m/m, and *N* is the porosity. Using this fonnula, the travel time is computed as:

# $V = 33$   $m/day(0.0005$   $m/m)/0.2$

# $V = 0.25$  *m/day*

The time required to move 660 m (2000 feet) is then roughly 7.3 years. Using the same formula, the transport time to WW-8, roughly  $1150$  m (3500 feet), approximately 12.7 years. This does not take into account the time required for movement through the vadose zone to the groundwater or any flow barriers that might be present underground in the aquifer. Use of the calculated site-specific average hydraulic conductivity of 30.4 m/day (92.7 ft/day) **(Table** 3) would result in similar fate and transport time.

As the overall groundwater flow direction absent the effects of the pumping wells is north-northwest, any plume from the pond would be expected to migrate in this direction, irrespective of influence from the on-site water supply wells. If this is the case, then chloride levels should significantly improve in the water supply wells located on the east side of the property (WW-8, WW-3) if WW-6 and WW-10 were shutdown. Based on the hydraulic conductivity discussed above, this should be noticeable in a relatively short time frame, on the order of six months or less. Theoretically, chloride levels in these wells would ultimately decrease to background. However, it must be noted that this scenario assumes that the lime ponds are the primary source, and does not take into consideration any chlorides migrating to the pumping wells from offsite to the east.

What could be fairly well established after the January 1999 event was that chlorides are migrating from the area around the Lime Ponds to the water supply wells. Questions still remained regarding other potential source areas and the level of chlorides which will be introduced at the water supply wells in the future.

**Table** 5 below illustrates the results of the August 1995 and January 1999 sampling events. Comparison between the chloride results from August 1995 and January 1999 indicate the following:

- 1.) The size and shape of the chloride plume had not appreciably changed.
- 2.) Outside the area of the greatest concentrations, the chloride results had not appreciably changed. In other words, background levels seem to be fairly consistent.

- 3.) Chloride concentrations in samples from the Lime Pond wells had decreased. **In**  some cases the decrease was dramatic. For example, measured concentrations from LP104 have decreased from 853 mg/l in August 1995 to 71.4 mg/l in January 1998. MW13, southwest of the Lime Ponds, also indicated a decrease of 98.2 mg/l from the August 1995 level.
- 4.) Chloride concentrations in samples from the water supply wells located in the area of highest concentration had generally increased since August 1995. WW-8 concentrations had increased 102.1 mg/I, from 343 mg/I in August 1995 to 445.1 mg/l in January 1998. Concentrations in WW-3 also increased. The exception was WW-10, which indicated a decrease of  $35.2 \text{ mg/l}$  since 1995. However, this change is within the expected range of values when accounting for sampling and laboratory variability.
- 5.) Chloride concentrations in the sample from WW-9, located outside the area of highest concentration, had decreased 35.6 mg/I since August 1995.
- 6.) Samples from the KDHE well, MW56, and ZL6 are all in the  $130 140$  mg/l range, within what might be considered background concentrations. These results indicated that there was no significant chloride contribution currently being introduced from a source east of the refinery.
- 7.) Of the twenty-five (25) wells sampled, the following wells indicated increases from 1995: WW-8, WW-3, ZL-1, and ZL-6. Of these four, changes in only the first three can be considered statistically significant. However, only seventeen (17) wells were sampled in 1995, so it was not possible to compare results for eight of the wells. Significantly,  $WW-6$  (351.4 mg/l) was one of the wells not

sampled in 1995. This well is located in the middle of the high chloride

concentration area.

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Sample Well	Chloride (nq/l) 1/99	Chloride $(nq/l)$ 8/95	Change 8/95 to 1/99
LP102	461.6	497	$-35.4$
WSW8	445.1	343	102.1
LP103	406.6	545	$-138.4$
WSW6	354.4	Na	Na
WSW3	351.7	296	55.7
ZL1	313.2	260	53.2
MW13	197.8	296	$-98.2$
ZL2	153.9	178	$-24.1$
WSW10	153.8	189	$-35.2$
MW25	142.9	Na	Na
MW15	142.9	154	$-11.1$
ZL6	137.4	118	19.4
MW56	137	Na	Na
KDHE well	131.9	Na	Na
MW43	120.9	Na	Na
MW8	109.9	Na	Na
ZL3	104.4	118	$-13.6$
MW42	104.4	Na	Na
LP101	101.7	343	$-241.3$
WSW9	82.4	118	$-35.6$
ZL4	76.9	83	$-6.1$
LP104	71.4	853	$-781.6$
MW9	60.4	Na	Na
LF9	54.9	83	$-28.1$
ZL5	22	95	$-73$
MWI	Na	204	Na

**Table** 5 - **Results of Chloride Sampling - January 1999 and August 1995**  Presented in descending order of 1/99 Chloride concentration

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# 4.4 Results of the November 1999 Field Work and Sampling Event

After analysis of the first two cvents had been completed, a third sampling event was scheduled which was designed to complement a comprehensive study with regard to the facility and the chloride source/movcment. This study addressed questions posed after analysis of the first events had been completed. These issues included:

- 1.) Chloride fate and transport.
- 2.) Chloride source areas (in particular from offsite).
- 3.) The concentration of chloride throughout the depth of the aquifer; ie; are the concentrations higher near the bottom or top.
- 4.) Influence of the pumping well screen location on chloride fate and transport.
- 5.) Influence of the aquifer stratigraphy on fate and transport of the chlorides.
- 6.) Influence of the vadose zone stratigraphy on the movement of chlorides into the aquifer.
- 7.) Future chloride levels in the pumping wells.

To address these items, it was necessary to perform a set of preliminary steps prior to sampling. First, in order to establish an accurate static water level elevation map, it was necessary to measure the top of casing (TOC), ground surface elevation (GSE), and horizontal locations for all wells to be used as part of the study. In addition, to determine the changes in chloride concentration with depth though the aquifer, it was necessary to drill a series of new monitoring wclls. The completion of these new wells produced a series of clustered deep and shallow wells screened such that samples of the upper and lower aquifer could be taken independently at essentially the same geographic location.

To detennine the extent of the chloride plume and to gain more infonnation regarding the potential source region to the east, a series of perimeter wells was also completed. Finally, an investigation designed to better identify both vadose zone and aquifer stratigraphic properties was undertaken. Details regarding these preliminary steps are included below:

#### 4.4.1 Determining the TOC and GSE for the Monitoring Wells

TOC and GSE data was generated for a total of 113 monitoring wells and pumping wells located on and around the facility. As part of the stratigraphy study, an additional 45 control points were obtained from the KGS Wizard database. This information did not include any of the refinery wells.

The elevation data was procured in a number of ways. Most of the elevations (horizontal and vertical) were measured in the field using a standard transit and stadia rod and/or global positioning system (GPS). For the sixteen new monitoring wells and the eleven landfann monitoring wells, a Kansas registered land surveyor was employed to ascertain the TOC and GSE elevations for these wells.

Once the elevation data for all the wells had been found, it was then possible to accurately determine the static water level elevation across the site. This information was used to help determine the static water level elevation used as part of the groundwater modeling analysis. This information was also used to construct an accurate static water level elevation map for the facility and all associated areas where monitoring wells were located. A copy of this static water levclmap is included below as **Figure 23.** 

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Figure 23 - Static Water Elevations (ft msl): Measured 11/16 & 11/17 1999.

# 4.4.2 Completion of the 16 New Monitoring Wells Onsite

In order to determine if there were any significant differences in the concentrations of chlorides between the upper and lower part of the aquifer, nested clusters of deep and shallow wells were completed at strategic points within the facility. The locations of these nested clusters were designed primarily to determine the extent of vertical and horizontal migration of chlorides from the area of the old lime ponds, a potential chloride source, to the pumping wells.

To accomplish this task, three "fencelines" were established at roughly equidistant locations between the old lime ponds and the pumping wells which intersected the area of chloride impact as determined by the results of the January 1999 sampling. **Figure 24** below shows the locations of the nested clusters. All of the fencelines were completed on the west side of pumping wells WW-3, WW-8 and WW-9, to minimize any impact that might be present from a source east of the refinery and concentrate efforts toward the lime ponds.

After careful study of logs for existing onsite monitoring wells, shallow or deep wells were drilled as close as possible to the existing wells to create a well pair, consisting of a shallow and a deep well. Six new shallow wells and 10 new deep wells were completed.



Figure 24 - Fenceline Locations for the Clustered Monitoring Wells

The screened interval of the shallow wells intersected, to the extent possible, the top of the saturated zone. Screen length in the new shallow wells was set at 6.6 meters (20 feet) and the wells were completed such that the top of the screened interval intersected the water table. For the deep wells, the screened interval was again set at twenty feet and the bottom of the screen intersected the base of the saturated zone as determined by examination of the drilling cuttings. Logs for these monitoring wells are included as **Appendix 2.** 

#### 4.4.1 a Field Drilling Procedures

Six shallow monitoring wells (between 36 and 40.6 meters; 110 and 124 feet deep) were installed with hollow stem auger methods. Ten deep monitoring wells (between 49 and 60 meters; 150 and 183 feet deep) were installed using mud rotary drilling methods. All wells were completed with protective construction at the surface. All wells were developed to reduce the suspended sediment in and improve the conductivity of the well with the surrounding aquifer.

Borings for the six shallow monitoring wells were advanced with 4-1/4" hollow stem augers with inside drill stem and pilot bit. Soil sampling was conducted by withdrawing the center drill stem, removing the pilot bit and replacing it with a 2" split spoon sampler which was then advanced into the soil ahead of the augers. Soil samples were collected every 1.6 m (5 feet) with the split spoon method.

The shallow wells are constructed with 2" Schedule 40, threaded flush joint, PVC riser and 0.010 inch factory mill slotted screen. All screen and riser was clean and in sealed containers from the factory. A  $0.010$  to  $0.020$  inch sized gravel pack, clean and

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bagged by the supplier, is placed to approximately two feet above the top of the screened intervals. To minimize caving from the sidewalls of the boring, the sand was placed in 1.6 meter (5 foot) "lifts" and the tremmie pipe was kept to within two feet of the existing filter pack. A two foot bentonite seal was placed above the gravel pack and allowed to hydrate before grouting the remainder of the well. The neat cement grout was pumped through a tremmie pipe and forced to the surface from the bottom of the open portion of each boring. Each shallow well is completed at the surface with a one meter (three feet) square concrete apron and a steel protective cover with a traffic guard.

Mud rotary drilling was utilized for the ten deep wells using a truck mounted drill rig with a 9-1/2" drill bit. Rotary-drilled borings are produced by pumping drilling fluid through the rotating drill string (drill stem and bit) where the fluid forces the cuttings away from the bit and to the surface through the annulus. The cuttings drop out in a settling pit where the drilling fluid is taken up by the pump and circulated back down the drill string. The bore-hole is advanced by lowering the rotating drill string the length of each successive drill stem. After each stem is drilled down the boring, water is circulated to allow cleaning of the hole prior to the subsequent connection of an additional stem. The process is repeated until the desired total depth of the boring is reached. The lithologic conditions at this site necessitated the use of a bentonite gel additive to the drilling fluid. The gel increases the viscosity of the drilling mud and minimizes caving of the sidewalls. Once each boring was completed the drilling mud was displaced with fresh water prior to setting the well. The deep mud rotary-drilled borings were advanced until shale was encountered.

The deep wells were constructed with 4" Schedule 40 flush-joint, threaded PVC casing with factory mill slot .020 inch PVC screens. Each deep well was constructed with a 6.6 meter (20 foot) screened interval at the bottom of each boring.

Once the screen and casing had been placed into the borehole a filter pack of 0.010 to 0.020 inch sized filter sand was tremmied into the hole. The sand was placed while keeping the tremmie pipe within two feet of the top of the filter pack surrounding the well screen and casing to minimize sorting and reduce the chance of bridging. Once the sand had been tremmied to approximately 0.7 m (2 ft) above the screened interval a bentonite seal approximately 0.7 m (2 ft) thick was placed around the casing. After the bentonite seal was allowed to hydrate, the borehole was force tremmied from the bottom up with neat cement grout. All the wells were completed with a one meter square (three feet by three feet) concrete apron, and a steel protective cover and steel cage at the surface. All wells were capped with an expanding J-plug type cap.

Monitoring wells were developed to enhance the flow of water from the formation into the well and to remove any particulate matter from the gravel pack and the well casing. The wells were developed by surging water through the well screen and filter pack using the rig and a surge block, then pumping until the water was clear and free of visible sediment. Generally, the shallow wells were developed with a Waterra actuated foot valve and a 2" Grundfos Redi Flo submersible pump. The deep wells were developed with a 3" steel bailer and/or by air-lifting using a 4" submersible pump. The on-site field geologist recorded the details of well design and construction for each

monitoring. Well.

## 4.4.1b Field Sampling Procedures for the Wells

During advancement of the borings for the 2" wells (shallow wells) soil samples were collected at 1.6 meter (5 foot) intervals for lithologic observations and headspace analysis. An experienced field geologist examined each sample for descriptions of, but not limited to, color, texture, grain size, grain shape, sorting, and moisture content. A portion of each sample was collected for field headspace reading with a Foxboro Organic Vapor Analyzer (OVA) Flame Ionization Detector (FID). The instrument was calibrated according to the manufacturer's recommendations. Soil samples were not submitted to a laboratory for further analysis.

During advancement of the borings for the 4" wells (deep wells) sediment samples were collected at 1.6 meter (5 foot) intervals for lithologic observations only. The samples were collected by placing a sieve at the top of the borehole allowing the returning drill fluid to pass through while retaining the cuttings. The field geologist described each sample for, but not limited to, color, texture, grain size, grain shape, and sorting. Field screening or laboratory analysis for organic vapors was not conducted on the soil samples from rotary drilling.

# 4.4.3 Chloride Fate and Transport

Completion of the fencelines of shallow and deep wells allowed spatial comparisons of chloride concentrations between the upper and lower parts of the aquifer to be made. In particular comparisons of chloride concentration data from paired deep and shallow wells made it possible to determine if the chloride preferentially migrated vertically through the saturated zone. It was felt that due to density differences between

fresh and brine water, the chlorides would tend to migrate toward the bottom of the aquifer with distance and thus chloride concentrations would be higher in the deeper wells. To test this hypothesis, the chloride concentrations in samples from the shallow and deep wells from the fenceline nearest to the lime pond wells (the source) could be compared to the chloride concentrations in each succeeding fenceline away from the source. If the results from the first fenceline indicated that the chloride levels were higher in the shallow wells and a reverse of this trend was noted with succeeding fencelines, this would indicate that the chlorides were migrating vertically through the saturated zone.

There was, however, an additional factor to consider when evaluating the chloride concentration data. The pumping wells should also be influencing the direction of chloride migration since the screened interval for these wells is roughly across the bottom 13 meters (40 feet) of the aquifer. Therefore, the pumping wells may preferentially cause vertical migration through the aquifer and add to any effect which would be caused by density differences between the fresh and brine water.

**Table 6** lists the results of the chloride sampling for all wells sampled, and also breaks the wells into deep and shallow categories. Descriptive statistics of the results do not indicate any statistically significant differences between the data sets. Figure 25 below graphically illustrates the concentration differences between the deep and the shallow wells. As can be seen from this graph, there is no significant difference indicated between chloride concentrations at the deep and shallow wells. However, this data was generated irrespective of location and only compares the overal1 average chloride concentration between the deep and shallow wells. In order to determine if vertical

migration is taking place with distance, maps illustrating spatial variations in deep and shallow chloride concentration were generated.



Figure 25 - Graphical Illustration of Chloride Concentration by Deep and Shallow Wells









NOTE: Wells in bold = Chloride greater than ppm

NOTE. Value for MW-21S is suspect and may be anomalous
**Figures 26 and** 27 below indicate that there does not appear to be a significant element of vertical migration across the site from the lime ponds to the water supply wel1s on the east side of the facility (water wells 3, 8, and 9). **Figure** 26 showing the shallow wells, does indicate a nose of higher chloride levels pointing in the direction of water wells 6 and 10, however migration does not appear to have promulgated beyond these pumping wells. This map also shows a "bulls-eye" of high chloride in the area of the tank fann, centered around MW-47. This is believed to be a shallow onsite source which is situated far enough away from the pumping wells so that this area has not been significantly effected by the wells.

**Figure** 27, mapping the deep well chloride concentrations, does not show a significant increase in chloride concentrations with depth beyond water wells 6 and 10. In fact, it can be seen that the deep and shallow chloride levels are consistent in the area of the refinery between the pumping wells. In both cases, the chloride levels are generally below 150 ppm. However, the deep well map does indicate a potential source from both the west and the east. The chlorides from the west appear to be migrating into water wells 6 and 10, while the chlorides from the east are migrating primarily into water wells 3 and 8. Water well 9, located on the southeast corner of the refinery, is currently unaffected by either of these potential sources.



Figure 26 - Shallow Well Chloride Concentrations; November 1999



Figure 27 - Deep Well Chloride Concentrations; November 1999

The results from these maps indicate that chlorides migrating from the east are captured by water wells 3 and 8, while chlorides migrating from the west are captured by water wells 6 and 10. These results do not indicate that there is any significant overlap between water wells with regard to the capture of the chlorides. It must be noted that some liberty was taken in the interpretation of the map due to limited data outside the study area.

**Figure 28** below is an ArcView generated map of chloride concentration, using all available data. Again, interpretation on the east side of the map must be viewed with some caution, due to lack of data in this region. Besides giving an overall view of the chloride concentrations, this map serves as a check for verification purposes of the information presented as part of **Figures 26 and** 27.



**Figure 28 - Chloride Concentrations (ppm), November 1999** 

The ArcView generated map of chloride concentrations above verifies the chloride results seen on the shallow and deep well maps **(Figures 27 and 28** above), indicating a separation zone bounded essentially by the pumping wells. This map also indicates that sources from the east and the west provide the majority of chloride water intercepted by the pumping wells.

**Figure 29** below illustrates chloride concentration contours and the static water level elevations as determined as part of the November 1999 sampling event. This figure does reveal evidence for a relationship between chloride concentration and static water level elevation, although there is not an absolute correlation. The relationship appears to be strongest on the east side of the facility. This indicates that the plume to the east is not a point source, as the pumping wells have an influence on the chloride plume. The chloride concentrations follow the changes in static water level elevation, which are caused to a significant degree by the pumping wells. This suggests that the plume to the east originates offsite, and is migrating towards the pumping wells due to the influence of the pumping wells on the water level elevations within the study area.

However, there appears to be very little relationship on the west side, which indicates that the areas of high chloride concentration noted on that side of the facility are not present due to offsite plume migration, but represent essentially point sources This can be stated as there appears to be little or no correlation with groundwater flow and chloride concentration on the east side as opposed to the west side. Therefore, it can be assumed that the areas of high chloride concentration found on the west side of that facility represent shallow point sources which the pumping wells have not yet significantly effected.



**Figure 29 - Static Water Levels (ft msJ) and Chloride Concentrations (ppm), 11/99** 

#### 4.4.3a Chloride Migration from the East

With regard to chloride concentration migrating from the east, there does appear to be an element of vertical movement evident. This can be seen by reviewing the chloride levels in the shallow and deep monitoring wells in that area. Deep monitoring well MW-l10D is located furthest east from pumping wells 3 and 8, followed by shallow well ZL-6 and finally shallow well MW-56, located approximately 16 meters (50 feet) from WW-8. The data shows that the highest concentration of chlorides is located in MW-IIOD, at 653.2 mg/l, folIowed by ZL-6 at 194.2 mg/l, and then MW-56 at 135.3 mg/1. A review of the concentrations taken from the pumping wells indicates much higher levels than noted in the shallow wells, with WW-3 at 447.2 mg/I and WW-8 at 517.9 mg/1. As noted earlier, the water wells are screened in the bottom 40 feet of the aquifer.

These results would indicate that on the east side of the facility, the highest chloride levels are found in the deeper section of the aquifer. This might also indicate that the pumping wells are preferentially drawing water from the deeper portion of the aquifer, which is probable given the screened interval of these wells.

Previous studies (Whittemore, 1997) have indicated that the chloride concentrations east of the facility increase to the east, reaching the highest levels approximately 3.2 km (2 miles) east of the refinery at the beginning of the Johnson Oil Field. In addition, the closed municipal landfill is located in approximately the same location and the Whittemore study indicated that it was also contributing to the elevated levels of chlorides found in this area.

Finally, the saturated thickness decreases with distance east of the refinery. The study referenced above found bedrock in the area of the landfill between 20 and 25 meters (62 and 75 feet) bgl, and saturated thickness at the landfill ranged from between 3.3 m (10 feet) on the east and 6.6 meters (20 feet) on the west. A well sampled approximately one mile west of the landfill, in the direction of the refinery, had a saturated thickness of approximately 10 meters (30 feet). Saturated thickness at the east edge of the study area is estimated at approximately 16 meters (50 feet) and was estimated at 43 meters (130 feet) on the west side of the study area, based on available well logs (Appendix I).

The information listed above favors vertical migration of the chlorides originating from the landfill/well field for the following reasons:

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- 1.) The reduced saturated thickness at the source indicates that less dilution will occur as the chlorides enter the groundwater, as the volume of water available for dilution is less in this area when compared to the saturated thickness west of the source area. Therefore, it would be expected that initial chloride concentration in the aquifer would be greatest here. Therefore, the density difference between the water at the source and the relatively fresh water to the west will be significant. **In** addition, the brine originating from the well field is also high in other naturally occurring material, such as bromide, sulfate, and sulfides, which will further increase the density of this water.
- 2.) Once the brine had contacted the bedrock, it would tend to remain at the bottom of the aquifer due to the density differences noted in (I) above. These same density differences would prevent significant mixing of the brine and fresh water as it migrated, except at the boundary zone.
- 3.) Assuming a relatively constant source renewal, as the brine moves laterally the surface area of the brine in contact with the fresh water increases, thereby promoting dilution of the chlorides with distance. However, the preferential location of the brine will remain at the bottom of the aquifer.
- 4.) As the brine reaches the pumping well radius of influence, the brine is preferentially moved into the area of the well and diluted by waters received at the well from the surrounding area within the radius of influence of the pumping well.

# 4.4.3b Chloride Migration from the West

The situation on the west side of the facility is much different that on the east side. Here saturated thickness is much greater, from 43 meters (l 30 feet) at the west edge of the study area to 56 meters (170 feet) or greater further west as the center of the McPherson channel is reached. In addition, there is no known offsite source that can explain the deep chloride concentrations or the shallow concentrations associated with monitoring well ZL-l.

It is possible that there is no offsite contaminant area as there is no data available outside the study area to the immediate west. Conversation with McPherson City officials has indicated that the chloride concentration of the water well located on the airport property approximately one mile northwest of the refinery exhibits chloride levels in the range of 30 to 50 ppm. However, this well is not located in an area that could be regarded as a source for the chlorides noted to the west of the refinery in any case. Comparison of the November 1999 chloride concentrations to the January 1999 concentrations for MW-15 and MW-13 reveal that the chlorides in both wells have significantly increased, on the order of 100 ppm or more. In January 1999 chloride concentrations in MW-13 and 15, were 197.8 ppm and 142.9 ppm, respectively. In November 1999 these concentrations were 294.2 ppm and 329.5 ppm, respectively. This represents an increase of 96.4 ppm for MW-13 and 186.6 ppm for MW-15.

Review of the August 1995 data shows that the chloride concentrations for MW-13 and 15 were 296 ppm and 154 ppm. These results suggest that MW-15 is the only well which has a recent and significant increase, as the 1995 level in MW-13 of 296 ppm is essentially the same value as the 294.2 ppm found in water taken from this well in

November 1999. Of note, the 1995 value from MW-15 (154 ppm) is essentially the same as the January 1999 value (142.9 ppm). Table 7 below illustrates these changes:

Table 7 - Chloride Concentration Differences for MW-13 and MW-15.

Well	<b>August</b> 1995	January 1999	November 1999
$MW-13$	296	197.8	294.2
$MW-15$	54	42.9	329.5

Analysis of Figure 28 shows that areal extent high chloride concentrations detected in the deep wells to the west is very limited in size, at least in the area studied. While it is conceivable that a plume less than 33 meters (100 feet) thick is being captured by WW-10 and moved across the site, it is more probable that the area indicated by the deep map represents a series of small source regions on the refinery itself that, when mapped, can appear to be a larger source potentially originating from offsite. Figure 30 shows a similar point source at and around shallow well MW-47 and potentially around MW-21S. In both cases, isolated areas of high chloride concentration can be noted. Both of these cases probably represent evidence for small source regions onsite, potentially the result of a past release at the surface. In the case of MW-21S, this is potentially an anomalous result. Samples taken for the source study (see Section 6.0) were used to verify these results.

The overriding feature of interest from the West is the shallow chloride concentrations in the area of the lime ponds and retention ponds. An area of elevated chloride concentration stems from the lime ponds and it seems to be moving in of direction  $WW-10$ . However, this shallow chloride plume does not extend beyond the area

of WW-10 Therefore, it appears that WW-10 effectively captures the high chloride waters and prevents this water from migrating further east.

With regard to the lime ponds potential to be a source region for chlorides, the chloride levels in monitoring wells  $LP-101$ ,  $LP-102$ , and  $LP-103$  have remained relatively consistent since August 1995. However, the result for LP-I04 has varied considerably from 853 ppm in August 1995 to 71.4 ppm in January 1999 to 559 ppm in November 1999. After the January 1999 event, it was postulated that the retention pond was not an appreciable source as the value in LP-104 had been reduced drastically from the 1995 levels. However, the November 1999 results do not support that hypothesis. Further study may be needed to verify what contribution the aeration pond makes to the overall shallow concentration of chloride in this area.

In addition, the values from ZL-1, on the west side of the retention pond and several hundred feet west of the lime ponds, have consistently indicated elevated concentrations of chloride. In fact, the concentrations of chloride have consistently increased each sampling event, from a value of 260 ppm in August 1995 to 313.2 ppm in January 1999 to 540.0 ppm in November 1999. This well is removed a significant distance from any refinery operations area with the exception of the cooling towers, which cool fresh water that has not heen in contact with process water. Therefore, a refinery source is unlikely although it is possible that a past release could have occurred in this area.

Groundwater movement in the area of ZL-1 and the lime ponds is primarily southeast, towards the pumping wells. Regional flow absent the effects of the pumping well is primarily north-northeast. This would mean that in order for the lime ponds to be

the source of the elevated chlorides found at ZL-l, a groundwater flow reversal would have taken place at some time in this area. As the pumping wells have been in use since World War Two, this scenario is highly unlikely. Therefore, the chlorides found in ZL-l are from the cooling towers, an old release, or an offsite source. In any event, they are migrating onto the refinery property and will add to the overall amount of chloride intercepted by the refinery pumping wells.

On a positive note, it does not appear that elevated concentrations of chloride are migrating offsite to the north. Deep wells 105D, 106D and 107D were completed as perimeter wells, and sampling results from all three of these wells indicate the chloride concentrations are at acceptable levels in their immediate vicinity.

#### **CHAPTERS**

# **GROUNDWATER MODELING**

## 5.1 Introduction

In order to understand the movement of potential chloride plumes and better establish source regions, it was necessary to develop a groundwater model, or numerical simulation, of the effects on the groundwater flow regime in the vicinity of the refinery by the pumping wells. The model selected for this process was Groundwater Vistas™, developed by Jim Rumbaugh. It is the first commercially available groundwater modeling program designed specifically to operate in a Windows environment and includes modules for MODFLOW, MODPATH, and MT3D, among others. This program also allows graphical interpretation of the results produced by these models.

#### 5.2 Modeling Parameters

The area of the groundwater model was set up on a 4000 meter (12,200 foot) square grid centered on the refinery **(Figure** 30), which represents the area of the chloride study area shown on **Figure 20** above. The grid cell size was set at 100 meters (305 feet). As noted above, the pumping wells are screened in the bottom 40 feet of the aquifer. In addition, the stratigraphic studies discussed above have shown significant differences in lithology between the upper and lower aquifers, Therefore, a three-layered model was developed which allowed vertical changes in groundwater movement and chloride concentration to be noted.

Groundwater Vistas allows changes in lithology and aquifer characteristics to be incorporated into the model. Therefore, changes in bedrock and static water level

elevations were incorporated into the model. In addition, it was possible to vary the hydraulic conductivity gradient across the study area in an attempt to simulate the changes believed to occur across the site and at the pumping wells in particular. Finally, measured levels of chloride concentration taken from monitoring and pumping wells located across the study area were introduced and plume maps were generated for the current situation and for future time steps. This included adjusting the boundary conditions to simulate the movement of a plume moving into the study area. The model used General Head boundary conditions, which allowed the greatest flexibility for adjusting the boundary conditions and simulated the conditions at the boundary with the greatest level of accuracy.



Figure 30 - Chloride Study Area with Grid

The modeling parameters used are given in **Table 8** below:

**Table 8 - Groundwater Modeling Parameters** 

Parameter	Value	Source
Grid size	4000 m	n/a
Spacing (cell size)	$100 \text{ m}$ (50m at wells)	n/a
Storativity (x-y)	0.05	Field Data
Leakance (ft/yr)	0.01	<b>KGS</b> Report
Recharge (ft/yr)	$6.85 \times 10^{-4}$	<b>KGS</b> Report
Hydraulic Conductivity X-Y (ft/day)	65 (E) to 165 (W)	Field Data
Hydraulic Conductivity Z (ft/day)	48	Field Data
Bottom Elevation (ft msl)	1280 (E) to 1340 (W)	Field Data
Static Water Level (ft msl)	1395 (E) to 1410 (W)	Field Data
Initial Chloride Concentration	varied based on well	Field Data





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**Figure 31** above is a W-E cross-sectional view of the model layers, bedrock elevation, and topographic elevation. The first (top) layer extends to the surface, however this does not effect the model results. The calculated gradient across the study area for the bottom/top of the layers are 1.6 x  $10^{-4}$  m/m and 7.9 x  $10^{-4}$  m/m (4.7 x  $10^{-4}$  ft/ft and 2.4 x  $10^{-3}$  ft/ft), from top to bottom, respectively.

# 5.3 Modeling Calibration and Sensitivity Analysis

The values listed in **Table 8** above were established after many "trial and error" runs were calibrated against actual field data. Calibration was performed against "target" wells using measured data and a sensitivity analysis was performed to establish the values which most closely approximate the measured values. Results of these analyses are included below.

#### 5.4 Modeling Results

The modeling results verified what had been postulated based on the observational results. **Figures32 (a-d)** below illustrates the drawdown and head contours for the study area given pumping rate and drawdown for the four supply wells listed in **Tables 1 and 2.** 

# **Figures 32 (a-d) - Modeled Drawdown (Ft MSL) and Head (ft bgl) for the Four Pumping Wells**



Figure 32a - Shallow Wells, Top Layer



Figure 32b - Mid Level, Layer 2



Figure 32c - Deep Wells. Layer 3

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Figure 32d - Cross Sectional View

The drawdown and contour interrvals illustrated on the figures above indicate that the radius of influence for the pumping wells has a greater extent on the east side of the pumping wells as opposed to the west side. This result is to be expected due to the decreasing saturated thickness on east of the pumping wells. **In** addition, it can be seen that the radius of influence for the wells overlap on the west side (between WW-3, WW-8, and WW-10) and the area of drawdown is located preferentially in the direction of the old Lime Ponds, a suspected local source of chloride contamination.

Using **Figure 32d** as a reference and the 4000 meter (12,200 ft) grid for distance, the area of water removed by the pumping well can be calculated along this cross-section. Calculations showed that the west side displacement was 108161  $m<sup>2</sup>$  $(1,163,954 \text{ sqft.})$  and the east side was 43267 m<sup>2</sup> (465,468 sq.ft.), for a total water displacement of 151461 m<sup>2</sup> (1,629,422 sq.ft.). Of note, the area indicated here is nearly twice as great on the west side as the east side. It can be seen on **Figure 32d** that there is a greater distance from the pumping well to the model boundary on the west side as opposed to the east, which accounts for some of this difference. However, a good portion of this difference can be explained as the saturated thickness declines on the east side of the facility.

As can be seen on the figures, the maximum drawdown is 13.8 meters (42.09 feet), and the minimum drawdown is  $9.8 \times 10^{-3}$  meters (0.03) feet. This is in good agreement with the measured figures for drawdown taken from the pumping wells. A minimum drawdown of 9.8 x  $10^{-3}$  meters suggests that the effective radius of influence for the pumping wells (at least on the west side) has been reasonably approximated by the area included as part of the chloride study. The figures show that the radius of influence of the pumping wells on the east side extends beyond the study boundary.

Figures 33–39 (a,b,c) below illustrate predicted changes in chloride concentration at the three levels studied for periods of 1,2, 5, 7, 10, 15, and 20 years from the present. The simulation was run using the chloride concentrations measured as part of the November 1999 sampling event, and assumes a chloride concentrations of 1200 ppm at the east boundary of the study area. The chloride concentration at the east boundary was an interpolation based on known concentrations from the onsite monitoring wells and the values taken from the report prepared by Whittemore (1997), which has been previously referenced.

# 5.4.1 Discussion of the Model Simulations

# 5.4.1 a Results from the One and Two Year Simulations:

The first two simulations illustrated in Figures 33 **and** 34 (a,b,c) show a logarithmic contour interval for the plume. As the "background" chloride concentration is assumed to be at zero for all of the study area except the boundary and the individual wells, the modeled concentrations can be theoretically modeled to a very low level, in this case to 0.01 ppm chlorides. In actuality, the background level is approximately 100 ppm, therefore the areal extent of the plume migration at these time intervals are portrayed by this contour interval

This initial logarithmic contour interval was chosen to represent not only the movement of the chloride plume (the 100 ppm interval), but the effects of the pumping wells on the movement of the plume and groundwater between the wells and the castcm

boundary. Although the chloride concentration shown on these figures is unrealistically small, the contours do indicate the relative distance from the well at which the groundwater begins to show definite influence from the pumping wells.

Analysis of the model results for these two events shows that substantial plume movement has progressed only a short distance from the initial boundary after two years. These results also show that the plume (represented by the 100 ppm line) is progressing essentially parallel to the boundary. This suggests that the pumping wells exhibit a relatively small influence on the movement of the plume at this distance. This is to be expected as influence of the pumping wells is a function of distance as it pertains to how much pumping is affecting the hydraulic gradients. As the plume gets closer the influence of the pumping wells will increase because the steepness of the hydraulic gradient will increase.

**Figures 33 and 34** also show a series of roughly circular lines around the northern half of the refinery. These lines represent the "zero" (or no chloride concentration) contour line for the individual wells on refinery property for which chloride levels were known and were included in the model. The end result, although somewhat difficult to ascertain, is roughly the extent of the chloride contamination onsite as determined by the available well data. This "zero" line is not absolutely accurate as not all of the well data is included at each level, however it does give some indication regarding the containment of the plume onsite due to the pumping wells.



**Figure 33 (a,b,c) - One year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.** 



Figure 34 (a,b,c) - Two year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.

#### 5.4.1b Modeled Movement after Five Years

All of the maps presented after two years feature a contour interval set at 100 ppm as opposed to the scale used on the previous maps. This new interval was chosen as the plume movement at five years and beyond has progressed to the point that a smaller interval is no longer necessary to show the plume movement and the influence of the pumping wells.

Review of Figures 35 (a, b, c) shows that the plume has migrated about halfway from the boundary to the pumping wells in the lower layer, and a distinct "nose" has developed as the plume comes under the influence of the pumping wells. As can be seen from these figures, the plume is moving more towards WW-3 and WW-8 than WW-9, indicating that the plume should impact WW-3 and WW-8 prior to effecting WW-9.

In the upper two layers, the plume has progressed further in the top layer than the middle layer. The top layer shows the plume progression almost as far as the bottom layer, while the middle layer shows much less progress. As pumping is confined to the lowest layer of the model, it is unclear why the model indicates such a marked difference between the top and middle layers. Discussion with other modelers have indicated that the effects of drawdown may be more pronounced than expected, thus producing this effect.

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**Figure 35 (a,b,c) - Five year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.** 

In addition to the plume movement noted at this time period, the "zero" line concentration contour can also be seen which surrounds nearly all of the facility. As before, this line represents the extent of the chlorides as described by the concentration in the wells shown on the figures and the extent of the 1200 ppm boundary on the east side. Please note that the extent of the 1200 ppm boundary is estimated; therefore the zero line as shown on the east must be interpreted with caution. However, as noted above, the zero line as shown for the remaining areas is consistent with the chloride results found as part of the November 1999 sampling event, as shown in **Figures 27 and 28** above.

# 5A.Ic Modeled Movement after Seven Years

At seven years into the simulation, the IOO ppm line has nearly reached the pumping wells. As can be seen from **Figures 36 (a,b,c)** the edge of the plume at the lowest layer is approximately 100 feet from the highway, and the nose of the plume has become even more pronounced. As with previous examples, the upper layers of the plume have progressed slightly slower than the lowest layer.

This simulation suggests that the nose of the plume initially migrates beneath refinery property on the west side of the highway slightly south of WW-8, between WW-8 and WW-9. This intrusion point is primarily a function of the combined influences of the three pumping wells on this side of the property. As a general rule, WW-9 has the highest pumping rate of the three wells, and this higher pump rate was incorporated into the model. This probably accounts for the initial contact at a location other than a pumping well.



Figure 36 (a,b,c) - Seven year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.

This intrusion point is also due to the initial position of the boundaries of the 1200 ppm chloride concentrations built into the eastern edge of the study area. It is probable that relocating or extending the 1200 ppm chloride level on the eastem edge would have some effect on the shape and interception point of the chlorides. For instance, if the 1200 ppm chloride boundary had been confined to the northem edge of the east border, the plume would have likely intercepted the property further north than the model shows. Likewise, if the extent of the 1200 ppm chloride level had been extended along the entire extent of the east boundary, the interception point and shape of the plume would probably have been closer to the boundary midpoint.

This situation is discussed because the actual extent of the chloride concentration on the east edge of the study area is not definitively known. It is possible that the chlorides are confined to an area smaller than that estimated, thereby producing a plume that is more narrow and focused than what the model currently indicates. However, it is also possible that the plume is wider than that estimated by the model, producing a larger plume than the modeled extent. In any event, the model results show that elevated levels of chlorides are steadily progressing onto the refinery property from the east. With regard to chloride concentrations in the remaining parts of the study area, the zero line has remained consistent when compared to the previous examples.

## 5.4.1d Modeled Movement after 10 Years

Review of the ten year simulation (Figures 37a, b, c) shows that the plume has reached pumping well WW-8 and the plume, represented by the 100 ppm line, is very near WW-9. Further review shows that the modeled concentrations arc in fair agreement with the actual concentrations taken from these wells as part of the two sampling events taken in 1999.

Sampling data from November 1999 indicated that the chloride concentration in WW-9 was at 117.7 ppm and the concentration in WW-8 was at 517.9 ppm. January 1999 concentrations were at 82.4 ppm and 445.1 ppm, respectively. The ten-year model results indicate that the concentration of WW-9 should be in the 100 ppm range, and the concentration in WW-8 should be approaching 400 ppm. These model results compare well to the concentrations noted as part of the January 1999 sampling event, and are in fair agreement with the concentrations found in November 1999.

Based on the model results and the chloride concentrations found in the wells, it can be surmised that the plume currently impacting the water wells was located at approximately the east boundary of the study area about 10 years ago. This is also in fair agreement with the calculated velocity of the plume discussed in the summary section above. In addition, as noted above in **Figure 6,** chloride samples taken from September 1986 did not indicate any evidence of elevated chloride impact. It must be noted that there is some variability in the model results as the initial concentration in the study area is assumed to be zero everywhere but the east boundary or at individual monitoring wells. However, some of this variability is accounted for by assigning a value of 100 ppm to the edge of the plume.



Figure 37 (a,b,c)- Ten year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.

## 5.4.1 e Modeled Movement at 15 and 20 Years

Review of Figures 38 **and** 39 (a,b,c) show that the chloride concentrations continue to increase over time at the pumping wells. However, the plume does not begin to significantly affect WW-9 until the 20-year simulation. In addition, it can be seen that the plume migrating onto the refinery property is contained by WW-8 and WW-9. Although the concentration lines continue to "bunch up" at and between these wells, the model does not indicate that migration occurs beyond these wells. Therefore, the model results suggest that the plume will be contained by the pumping wells as long as the wells are in use. In fact, the model results suggest that at least one of the wells could be taken out of service and still maintain control of plume migration.



**Figure 38 (a,b,c) - Fifteen year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary.** 



Figure 39 (a,b,c) - Twenty year simulation of chloride concentrations, 1200 ppm chloride concentration at the East boundary

#### 5.4.2 Summary Discussion of the Model Results

As illustrated by the figures above, the model indicates a chloride travel time from the eastern boundary to water wells #3 and #8 of approximately 10 years. It can be seen that chlorides arrive at the pumping wells above background levels in the lowest layer (Layer 3) initially, followed by the top layer (Layer 1) and finally the middle layer (Layer 2).

Chlorides arrive first at the lowest layer for a number of reasons. As discussed above, the preferential pathway for chlorides originating east of the facility is along the bottom of the aquifer. This is due to the decreased saturated thickness near the source, density differences between fresh water and brine, and the location of the screened interval at the bottom 13 meters (40 feet) of the pumping wells. In addition, stratigraphic studies have shown that that aquifer is coarsest in the lower levels, indicating that this may be a zone of higher hydraulic conductivity. Therefore, water high in chloride migrating to this zone may also move more rapidly through this zone.

To verify the model results, travel time from the east boundary to the well was estimated using the following formula for average linear velocity (taken from Heath, 1991):

## $V = K I/N$

Where *V* is the velocity, in m/day, *K* is the hydraulic conductivity, in m/day, *I* is the hydraulic gradient, in ft, and  $N$  is the porosity. Using this formula, the travel time is computed as:

# $V = 24.6$  m/day(0.004.92 x 10<sup>-1</sup> m/m)/0.2

# $V = 0.18$  m/day (0.56 ft/day)

This equates to approximately 67.2 meters/year (205 feet/year), or 672 meters (2050 feet) over ten years. The distance between the eastern edge of the boundary and WW-3 and WW-8 is approximately 787 meters, or 2400 feet (after **Figure 21** above). The hydraulic conductivity (K) value of 24.6 m/day (75 ft/day) used above is based on the known conductivity changes across the entire site and approximates the average conductivity between the eastern edge of the boundary and the pumping wells. The value used for hydraulic gradient  $(I)$  also represents the overall hydraulic gradient across the study area. Given the potential range of input parameters, the difference between the modeled velocity and the calculated velocity is reasonable and does a fair job of validating the results of the model simulation.

Elevated chloride levels also reach the pumping wells in the top layer and the middle layers of the aquifer. The initial chloride concentration of 1200 ppm set at the east boundary of the study area was constant throughout the three layers of the model. This may not be strictly representative of the actual vertical concentration of chlorides at the boundary, however no data *is* available for the upper two layers at the boundary. *This*  represents a conservative scenario and may overestimate the volume of the source.

What can be surmised to a degree from the upper layers of the aquifer is the transport rate absent the effects of active pumping in the upper two zones, as the water wells are screened in the lower portion of the aquifer. For the model, the pump rate was set at zero in both the upper and middle layer. Based on this, it can be postulated that transport of the chlorides in the upper layers is a function of the observed gradient. although the gradient is significantly effected by the pumping wells. This is of particular significance in the study area due to drawdown by the pumping wells across the study area.
#### CHAPTER 6

#### **THE CHLORIDE SOURCE IDENTIFICATION STUDY**

#### 6.1 Introduction

Following completion and analysis of the November 1999 sampling event, a final phase of the study was undertaken to further define the source(s) of the chlorides found as part of the sampling. This effort also served as verification of the previous sampling results for a selected group of monitoring wells **(Table 10)** that exhibited potentially anomalous results or were located in areas of particular signi ficance for the overall study.

**In** order to accomplish the source identification, Dr. Don Whittemore (KGS) was contacted. Dr. Whittemore has pioneered efforts in this area and has been employed in chloride source identification studies by the KGS and KDHE, including a 1997 study in the McPherson area. Identification of the chloride source(s) followed the geochemical methods of Whittemore (1984, 1988, 1995, 1997). These methods primarily include plots of the constituent mass ratios bromide/chloride and sulfate/chloride vs. chloride concentrations, and points for the water sample data and curves for the mixing of different source waters.

Each mixing curve is calculated using an algebraic equation for conservative mixing of two end-point waters. Conservative mixing equates to the simple mixing of water without chemical reactions that could alter the concentrations of one or both of the constituents. The intersection of the two mixing curves can be determined graphically or by solving for simultaneous algebraic equations. Logarithmic scales are used in the

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graphs to produce a more even distribution of points for large ranges in concentration than would be noted utilizing linear scales.

#### 6.1.1 Results of the 1997 Whittemore Study

As brietly introduced in Section 4.0 above, a previous study undertaken by Whittemore (1997) indicated that the chloride concentrations east of the facility increase to the east, reaching the highest levels approximately 3.2 km (2 miles) east of the refinery at the beginning of the Johnson Oil Field. In addition, the closed municipal landfill is located in approximately the same location and the Whittemore study indicated that it was also contributing to the elevated levels of chlorides found in this area.

**Figure 40** (after Whittemore, 1997) below illustrates the sampling locations utilized as part of the 1997 study. As can be seen on this figure, samples were taken from the area of the Johnson Well Field and the old Municipal Landfill, in addition to samples of domestic wells located between the well field and the refinery. **Table** 9 (after Whittemore, 1997) below shows the chloride concentrations noted in samples taken from these locations. This table also indicates the concentrations of sulfate and bromide, along with the bromide/chloride and sulfate/chloride ratios for the waters.

The 1997 study revealed that brine waters originating from the Johnson Well Field had a Br/Cl ratio in the range of  $43.8$  to  $45.3$  (after multiplying by 10,000). Whittemore reported that oil field brine was the primary saltwater source for all of the points sampled as part of the study, and the ratios ranged from 28.6 to 42.1 (after multiplying by 10,000). For samples taken in areas outside the intluence of the municipal

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landfill. Whittemore reported that the percent of total chlorides in the groundwater samples attributed to oil field brine was no less than 84%.

**Table** 9 shows that chloride concentrations in the Bowman and Tim Shaw domestic wells, located roughly 1.6 km (one mile) east of WW's 3, 8, and 9, were 2530 ppm and 3130 ppm, respectively. The John Shaw well, located approximately 2.4 km (1.5 miles) from the pumping wells, indicated a chloride concentration of 1566 ppm, and MW-7 located north of the John Shaw well but south of the old Municipal Landfill, showed a chloride concentration of 2360 ppm. Samples of the lease brine taken approximately 2 miles east of the water wells indicated chloride concentrations in excess of 100,000 ppm. The results of the initial study clearly show that brine from the Johnson Well Field has impacted the groundwater and is moving towards the refinery pumping wells.



### Figure 40 - Locations of the Sampling Points



### Table 9 - Results of the 1997 Whittemore Study

Map locator number	Sample identification	Location	Collection date	Collection time	Date received	Date analysis reported	KDHE. lab number	Collector
16	Geoprobe $#1$	19-03W-35CBBB	3/6/97	11:45	3/7/97	3/26/97	701104PT	Pover/Dallen
18	Geoprobe $#3$	19-03W-35CBCB	3/6/97	15:45	3/7/97	3/26/97	701106PT	Poyer/Dallen
	$MW-7S$	19-03W-34DAAB	10/1/96	14:45	10/2/96	10/8/96*	700582PT	Medina
	$MW-7D$	19-03W-34CDAB	10/2/97	14:50	10/2/96	$10/8/96*$	700580PT	Medina
20	Bowman domestic well	19-03W-34CCC	9/12/96		9/13/96	11/3/97	700472PT	Medina/Poyer
13	John Shaw domestic well	19-03W-34CDD	10/1/97	15:45	10/2/96	10/8/96*	700581PT	Medina
21	Fred Johnson stock well	20-03W-03 ADD	11/25/38					

**Table** 9 - **Results of the 1997 Whittemore Study, Continued** 



- 6.2 Discussion Regarding the Current Source Identification Study
- 6.2.1 Introduction

The source identification study was undertaken in an attempt to add further verification to the sampling and modeling results with regard to source(s) of the chlorides impacting the water wells at the facility. As noted above, the 1997 Whittemore study had produced a characteristic BrlCI ratio that could be used for comparison against the results found from groundwater sampled from the wells used in the current study.

One potential drawback to the method is that since the refinery processes crude oil, and the chlorides found onsite might have been brought onto the facility via the crude oil processed, it might be difficult to distinguish between the Johnson oil field brine and the brine originating from crude processed at the facility. This assumes that the chemical characteristics of all oil field brine is similar to that found from samples of the Johnson oil field brine. In addition, it is known that the refinery has processed crude from the Johnson oil field for a number of years. Fortunately, the results of the sampling did indicate marked differences between the chemical characteristics of the groundwater from the sampled wells, which provided for effective comparison to the Johnson oil field results.

#### 6.2.2 Results of the Current Source Identi fication Study

**Table 10** below illustrates the monitoring wells from which groundwater samples were taken for purposes of source identification. As noted above, these wells were also chosen as verification for previous sampling results.



### Table 10 - Source Identification Monitoring Points and Results (Based on Whittemore, 2000)



### **Table 10 - Source Identification Monitoring Points and Results (Based on Whittemore, 2000), Continued**

Review of the information contained in the table reveals differences between the chemical composition of groundwater taken from the various sampling points. These differences are discussed in further detail below. As can be noted upon review of the information discussed below, the results of the source identification study strongly support the modeling results and the field studies.

The source study analysis indicated that monitoring well MW-11 OD and water well #8 have the highest Br/Cl ratio correlation when compared to the ratio for the Johnson well field found as part of the 1997 Whittemore study. In addition, monitoring well ZL-6, a relatively shallow well located east of water wells 3, 8, and 9, also shows evidence of oil field brine contamination. The water from ZL-6 indicates mixing of oil field brine with fresh water, which is what the model results predict for a shallow well located in the area of ZL-6. The concentration of oil field brine noted in ZL-6 is most probably due to the influence of the cone of depression caused by the pumping wells, and associated mixing that will occur along the boundary between the brine and fresh zones.

The source study also showed that the shallow point sources found on the facility, including the area around the old lime ponds, indicated distinct chemical differences from the Johnson well field brine samples. The  $Br/C1$  ratio is distinctly different for the shallow point source regions, and there is also a difference in the sulfate/chloride ratio, the iodide concentration, and the nitrate concentrations. While the study results do not definitively point to a source for these shallow onsite samples, the results do show that Johnson oil-field brine is not the primary source.

After review of the results from the current study, Whittemore divided the monitoring wells into five different groups, based on the chemical constituents found in

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the groundwater from each well (personal communication, 2000). These groups are discussed below:

### **Group 1: Waters with a substantial chloride contribution from oil-field brine Well numbers WW#8 and 1100**

Whittemore reported that groundwater taken from these wells exhibits a Br/Cl ratio similar to the groundwater contaminated by the Johnson oil field brine to the east of the NCRA area. The sulfate/chloride ratios are low as would be expected for a mixture of fresh groundwater in the Equus Beds aquifer with small amounts of oil brine. The inorganic iodine content from both samples is low. The cation and Na/Cl ratios for these samples indicated that cation exchange has occurred (adsorption of Na and release of Ca and Mg into solution) during the migration of the Na-Cl chemical type of saltwater source (oil brine) into the Equus Beds aquifer.

MW-llOD is a deep well and is located furthest east. As previously noted, all indications are that this well should demonstrate the most pronounced impact from the Johnson well field brine. **In** addition, model results show that WW#8 is the water well initially impacted by the plume originating from offsite to the east. Therefore, the chemical results for these two wells are in agreement with all other elements of the study.

# Group 2: Waters with a substantial chloride contribution from oil-field brine **and a small contribution from an additional source low in sulfate, nitrate, and iodine**

**Well numbers W\V#3, 150, ZL-6** 

These samples fit the mixing of oil-brine with fresh ground water in the Equus Beds that have been affected by a small additional chloride source. The waters have relatively low sulfate/chloride ratios and low nitrate and iodine concentrations. WW#3 and ZL-6 are located on the east side of the facility. ZL-6 is a shallow well east of WW#3 but west ofMW-110D, while WW#3 is one of the water wells located on the east side of the facility. The chemical results indicate that ZL-6 and WW#3 have been impacted by oil field brine, however not to the extent that WW#8 and MW-110D have been. The modeling and sampling results support this hypothesis. MW-150 is a shallow well located on the west side of the facility. The chemical results indicate that the contamination found in this well may consist of oil field brine, however given the location and the other chemical constituents, it is unlikely that the source is the Johnson oil field.

## **Group 3a: Water with a mixed chloride source that could include some oil brine contamination and sorne other saline water that has a high sulfate concentration.**

#### **Well number LP-I03**

This sample fits the mixing of oil field brine, saline water with a high sulfate concentration, and fresh ground water in the Equus Beds. The water has a substantially higher sulfate content and sulfate/chloride ratio than found in Group 2 but otherwise is similar based on low nitrate and iodine concentrations. Monitoring well LP-I03 is a shallow well located in the area of the old lime ponds, a suspected chloride source. Chlorides from this well are postulated to originate from refinery sources as water in the

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old unlined lime ponds migrated through the vadose zone to the aquifer. The chemical results noted above support this hypothesis as a higher sulfate concentration would be expected for water that had been processed by the refinery, as sulfur and sulfur compounds are byproducts of the refining process.

It appears, therefore, that the source for the chlorides found in groundwater samples from this well originate primarily from brine waters processed by the refinery. However, it must be noted that the refinery uses water from the water wells that have been impacted by brine from the Johnson well field. Until the ponds were taken out of service in 1994, much of the process water would have been ultimately directed to the lime ponds. While the water wells have been recently impacted when compared to the total time the lime ponds were in service, it is possible that a portion of the water from this well and the other lime pond wells may include brine from the Johnson well field.

# **Group 3b: Water with a high nitrate content that has a mixed chloride source that could include some oil brine contamination and some other saline water that has a high sulfate concentration.**

#### **Well number ZL-t**

This water fits the mixing of oil-brine and another saline water with a high sulfate concentration with fresh ground water in the Equus Beds. The water has a high nitrate content and a substantially higher sulfate content and sulfate/chloride ratio than the waters in Group 2 but has a low iodine concentration.

Monitoring well ZL-l is located away from all refinery processes except the cooling towers, which are used to cool process water from the refinery. It is also located

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immediately adjacent to a farm field currently producing cash crops. The relatively high nitrate concentration is undoubtedly due to the proximity of the fann field and the use of a nitrogen containing fertilizer (such as ammonia or ammonium) on the field. The incidence of high sulfate concentrations may be attributed to the process water, as there is some loss due to spray as water is taken through the cooling towers. As in the case of the lime pond wells, a percentage of the chlorides may originate from Johnson oil field brine that was introduced to the refinery process system and was subsequently introduced into the groundwater by the infiltration of spray from the cooling towers.

## **Group 3c: Water with high nitrate and sulfate contents and elevated iodide concentration.**

#### **Well water LP-I02**

This sample has some similarities to water from well ZL-l but has a much higher iodide content. This suggests that it has been affected by waters similar to Group 4a or 4b. As with the previous lime pond well samples, this sample had elevated sulfate levels and therefore the source is probably refinery process water.

### **Group 4a: Waters with a relatively high iodide content and bromide/chloride ratio and low sulfate and nitrate concentrations.**

#### **Well numbers MW-47 Sand 20 0**

Well water MW-47S has the highest *Br/Cl* ratio of all groundwater samples; the ratio is substantially greater than that of the Johnson oil field brine. Water from 200 has a much smaller chloride content than MW-47S, however it has a similar iodide content.

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Whittemore proposed that water from MW-20D could possibly be a mixture of water from the MW-47S area with fresh ground water from the Equus Beds. While this is possible, based on groundwater flow, it is felt that based on the locations of the respective wells it is more probable that these wells represent separate point source releases of similar material at some point in the past.

One of the distinguishing differences between Johnson oil field brine and brine from shallow sources on the refinery property was the presence of elevated levels of iodide in the refinery samples when compared 0 the Johnson oil field samples. The source of the high iodide content is unknown, as there are no refinery processes that use iodine or iodine compounds. It is possible that brine from other oil fields contain a higher iodide content, however this is speculation. Additional sampling and analysis of brine from other oil fields supplying crude to the refinery would be necessary to determine the true source of elevated iodide concentrations.

## **Group 4b: Waters with a relatively high iodide content and low bromide/chloride ratio and low sulfate and nitrate concentrations.**

#### **Well numbers MW-21S and MW-53D**

Well water MW-21S has the largest iodide concentration of all the ground water. Groundwater from MW-53D could possibly be a mixture of water from the MW-21S area with fresh ground water from the Equus Beds. MW-21S is a shallow well and MW-53D is a deep well; the wells are located approximately 16.4 meters (50 feet) apart. These wells are located close enough together so that the levels of iodide in the respective wells is governed by the migration rate of iodide through the aquifer from shallow to

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deep. As MW-21S is the shallow well and the source here is believed to be an onsite point source release, concentrations are higher in the shallow well. Continued monitoring of the wells over time should show a reduction in the difference between shallow and deep iodide concentrations.

## Group 5: Water with a relatively low Br/CI ratio, a high nitrate content and a **moderately high sulfate concentration.**

#### **Well water LP-I04**

Whittemore reported that the *Na/Cl* ratio from this well indicated saline water had migrated into the Equus Bed aquifer that was predominantly Na-Cl in chemical type. He postulated that cation exchange has occurred during leaching and transport that has decreased the Na/Cl ratio over time. The Ca/Mg ratio is higher than for waters from wells 8D and 110D, suggesting that a saline water with an additional calcium source is included in the mixture affecting the ground water.

Monitoring well LP-104 is one of four wells completed in the area of the old lime ponds. As such, it would be expected that the chemical characteristics of this well would be similar to that found in samples from LP-102 and LP-103. With regard to the sulfate content of LP-103 and the nitrate content of LP-102, this is true. However, the *Br*/Cl ratio for LP-104 is approximately half that found in the other lime pond wells. One potential explanation for the differences between LP-l 04 and the other lime pond wells is that LP-104 is located closest to the presently used large retention pond. Water softening of the refinery process water that is sent to the retention pond prior to discharge to the receiving stream will result in higher calcium and magnesium concentrations in the ponds

than would otherwise be naturally occurring. It is reasonable to suggest that infiltration from the retention pond has caused the differences in groundwater quality noted from this well. In particular, the higher Ca/Mg ratio and the calcium noted in groundwater from this well could stem from water supplied by the retention pond.

Whittemore had also suggested that this water could be affected by infiltration of water with a low bromide/chloride ratio such as wastewater from the municipal treatment plant of McPherson. The retention pond has many of the same characteristics as the municipal waste water, (such as drinking water that has been softened) as it receives process water that has been treated by the onsite NCRA water treatment system.

#### **CHAPTER** 7

#### **SUMMARY & CONCLUSIONS**

A variety of studies have been undertaken in an attempt to discern the source location, fate and transport of the elevated concentrations of chlorides that have been noted in monitoring wells and pumping wells on and around the refinery facility. These studies included sampling of the monitoring wells, spatial analysis of chloride concentration data, investigation of the stratigraphic and lithologic properties of the vadose and saturated zone (the Equus Beds aquifer), numeric modeling of ground water flow beneath the study area, and geochemical source identification analysis for the potential sources of chlorides. These studies have revealed that the chlorides noted in ground water in and around the facility probably originate from two separate sources. The principle source originates from the Johnson Well Field approximately 2 miles east of the refinery. The remainder of the elevated chloride concentrations originate from localized shallow point sources primarily within the refinery operations area and tank farm. These point sources are probably the result of past releases on the refinery property and do not represent offsite sources.

Numeric modeling studies indicate that the four onsite water supply wells are effectively controlling the extent of the chloride migration by capturing the plume(s) as it/they move into the water supply well radius of influence. As the radius of influence for the wells overlap, the wells can be regarded as a single system for purposes of plume migration control and remediation.

Study of spatial patterns of chloride concentration and numerical modeling results have shown that the shallow point source areas are being controlled by  $WW-10$ ,

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located essentially in the middle of the refinery. Point sources controlled by this well include the area around the old lime ponds, and area in the refinery operations area roughly bounded by MW-21, MW-15 and MW-13. Another point source located in the tank farm, in the area around MW-47, appears to be within the radius of influence, but far enough away so that its migration is controlled to a lesser extent than the other point sources are.

The source originating from the Johnson Well Field is being controlled by the pumpage of water wells located on the east side of the facility (WW's 3, 8, and 9). Numerical studies have indicated that even after twenty years of migration, the east wells will continue to capture all of the chlorides migrating onto the refinery property. In addition, the numerical studies have shown that the chlorides will not migrate offsite, due in large part to the influence of the pumping wells, which divert the regional groundwater flow to the pumping wells. The net effect of the pumping wells has been to create an area on the refinery property between the pumping wells that is essentially free of elevated chlorides. Study has indicated that as long as the existing water wells continue to pump, this condition will remain stable.

It is believed that the overall quality of the water captured by the pumping wells will continue to decline over time. The practices that resulted in the shallow onsite sources have been largely eliminated and there is no longer any appreciable influx of brine from activities associated with the Johnson Well Field.

The Johnson Well Field and the Refinery have both been in operation since the 1930's. Environmental regulations designed to curtail sources of contamination from entering the groundwater have been in place only since the mid 1970's. Therefore, source

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infiltration occurred for at least 40 years before the environmental regulations and a heightened sense of awareness regarding potential damage to the environment was recognized, resulting in the implementation of source reduction measures.

Based on this and the results of the groundwater sampling, it is believed that these sources are generally large enough and there is enough source material trapped in the vadose zone that can migrate to the groundwater that no appreciable reduction in source renewal to the groundwater can be expected in the near future.

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### Appendix 1 Well Logs













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#### WELL INFORMATION

# Layne-Western Co. Inc.







#### 11. GRAVEL

Size  $1/8 \times 1/2$  Miles Tons  $18$ 

#### 12. SEALING CASING



- 13. WELL DIMENSIONS
	- (From Top of Inner Casing to Bottom o Well)
	- B. Height of Inner Casing........... 2' 6".......... (Above Ground Level)
	- C. Distance to Top of Gravel.........99.'............. (From Ground Level)
	- D. Diameter of Drill Hole ... 3.0."... s... 38."...

Comments Reverse rotary

#### 11. PUMPING TEST

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#### **Resk DRINK**

Permanent pump



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C. Length of airline .....140.......Ft. from top of casing.



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# Appendix 2 Water Well Pumping Data

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## Appendix 3

Water Well Data

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**Communication** 

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Analysis of Chloride Plume Migration Based on Aquifer Characteristics, Water Well Pumping Rate, Soil, and Stratigraphic Properties Title of Thesis/Research Project

Signature of Graduate Office Staff Member

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